

Incorporating Soil Nonlinearity into Physics-Based Ground Motion Simulations

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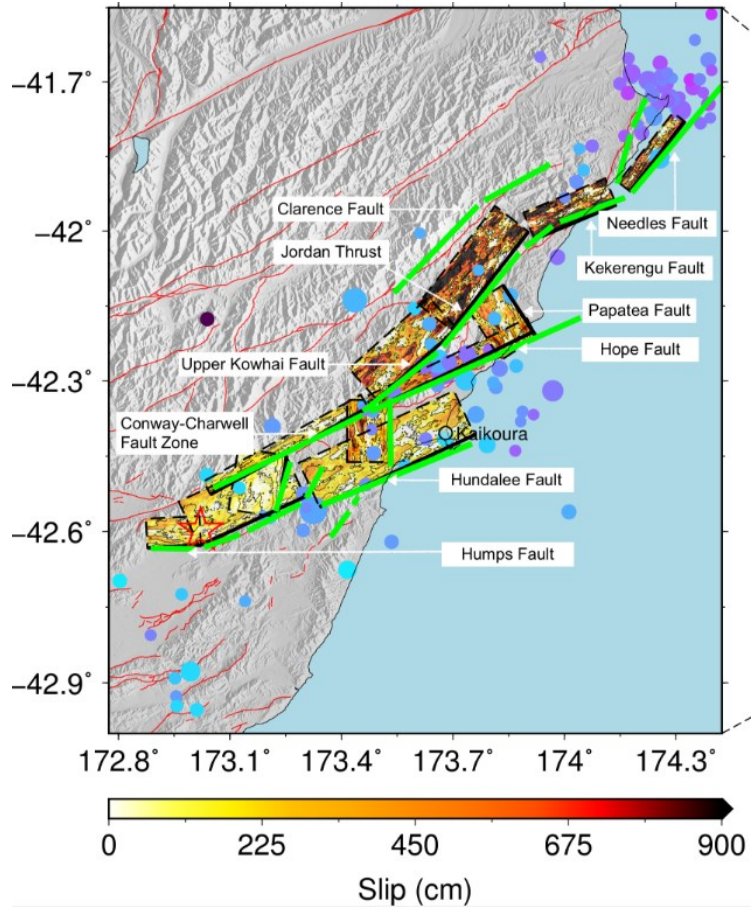
Co-Advisor: Misko Cubrinovski

Co-Advisor: Liam Wotherspoon



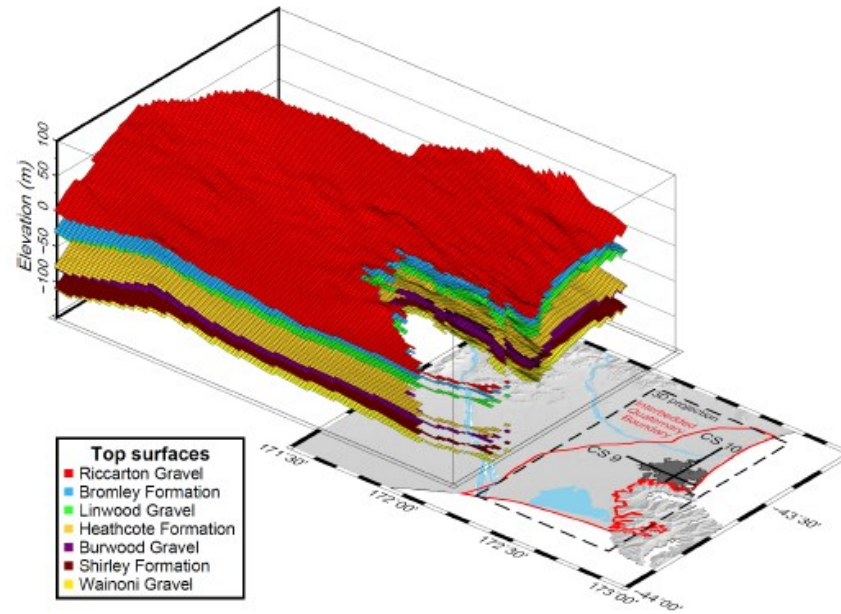
QuakeCoRE
NZ Centre for Earthquake Resilience

Seismic Source



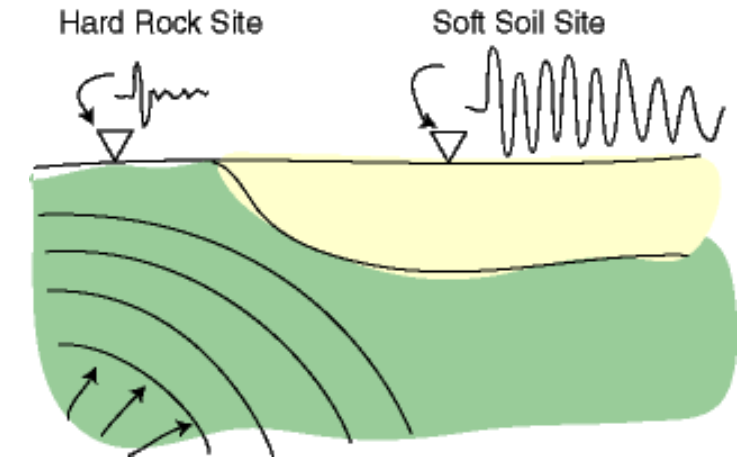
Bradley et al. (2017)

Source-to-Site Path

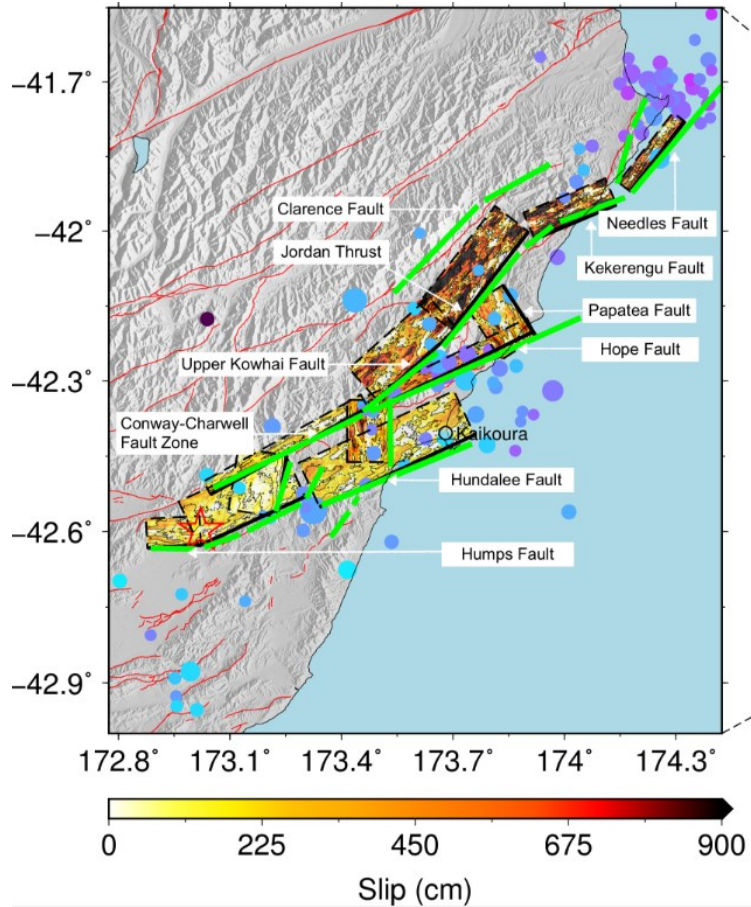


Lee et al. (2015)

Site Effects

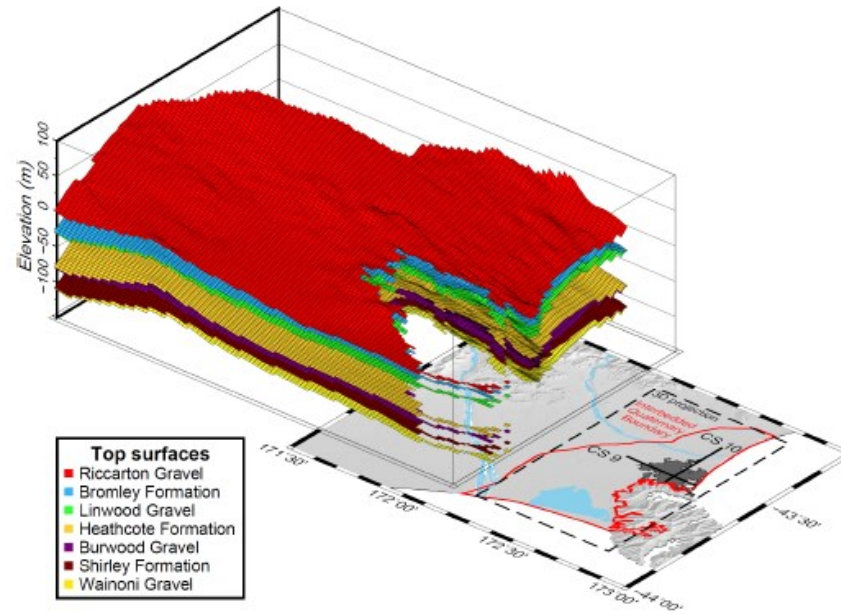


Seismic Source



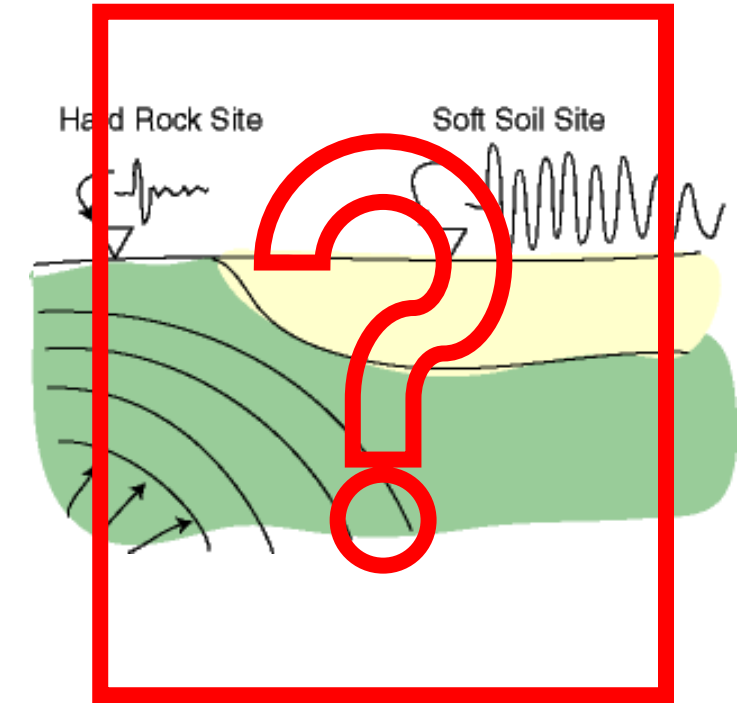
Bradley et al. (2017)

Source-to-Site Path



Lee et al. (2015)

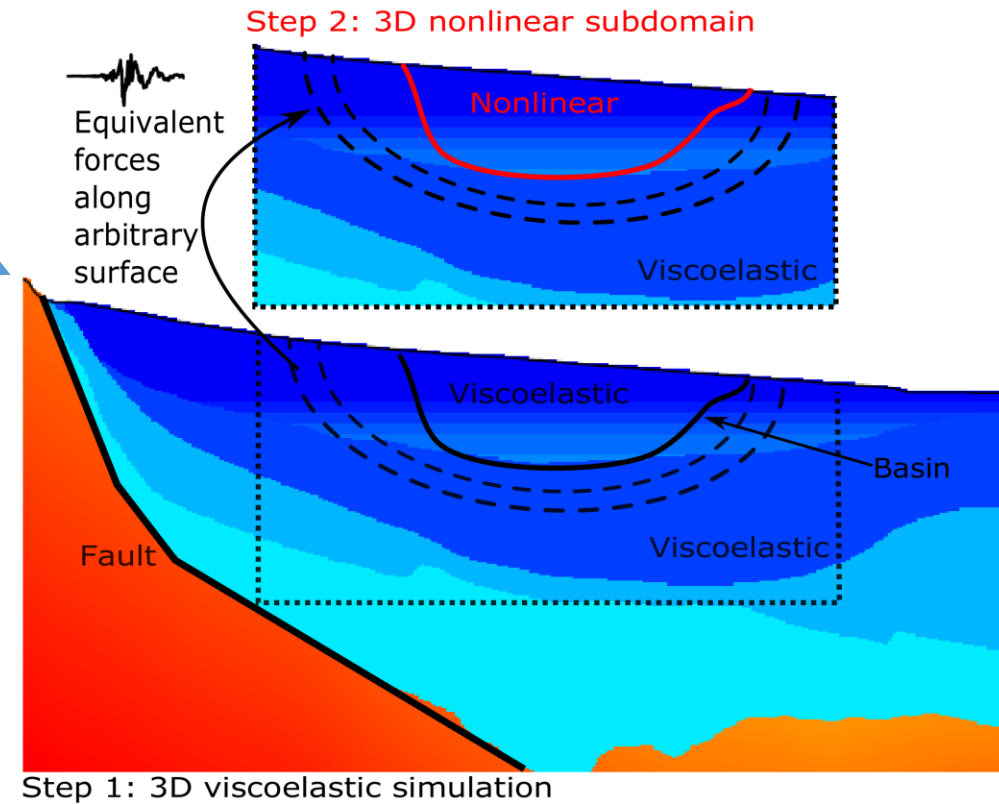
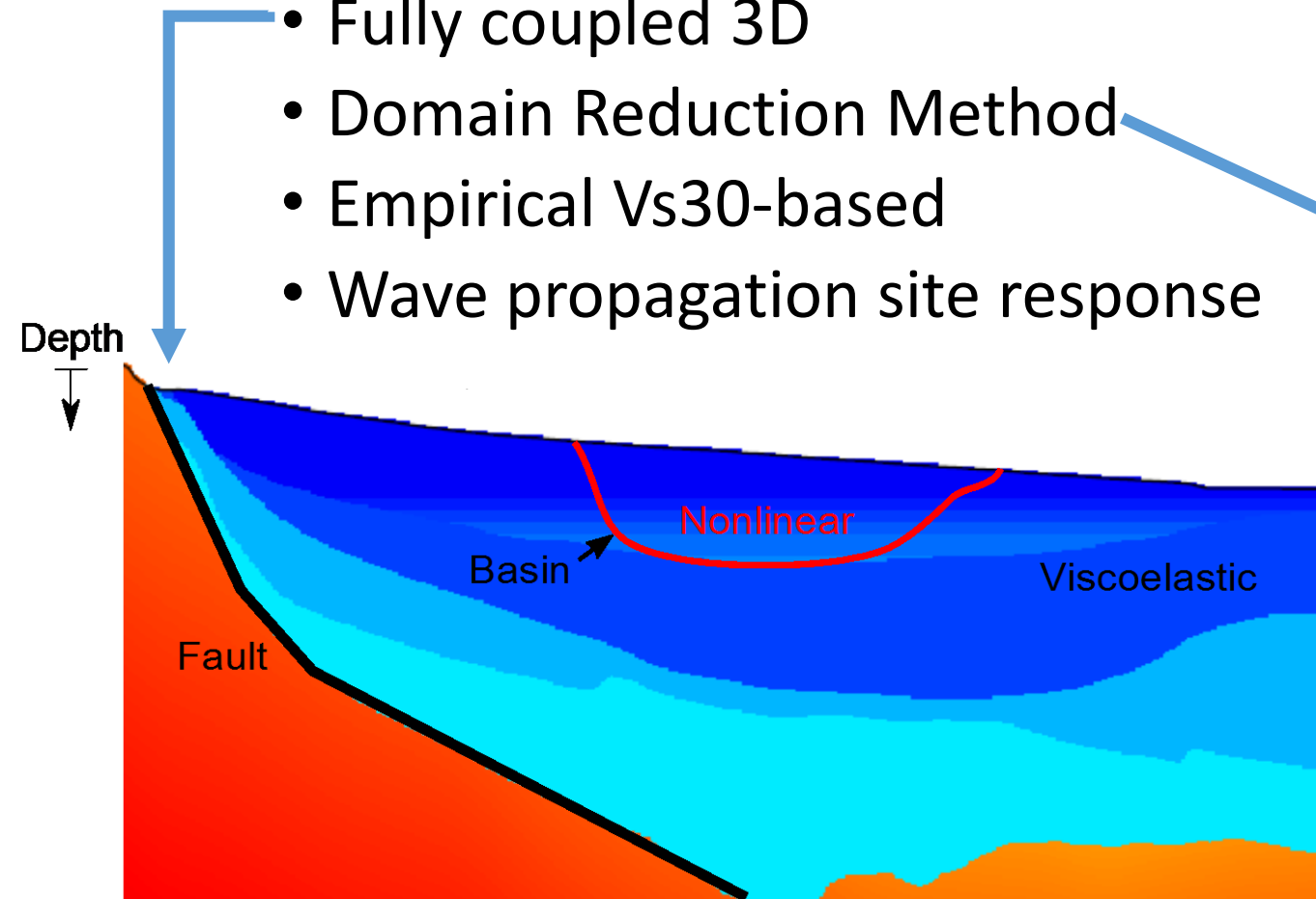
Site Effects



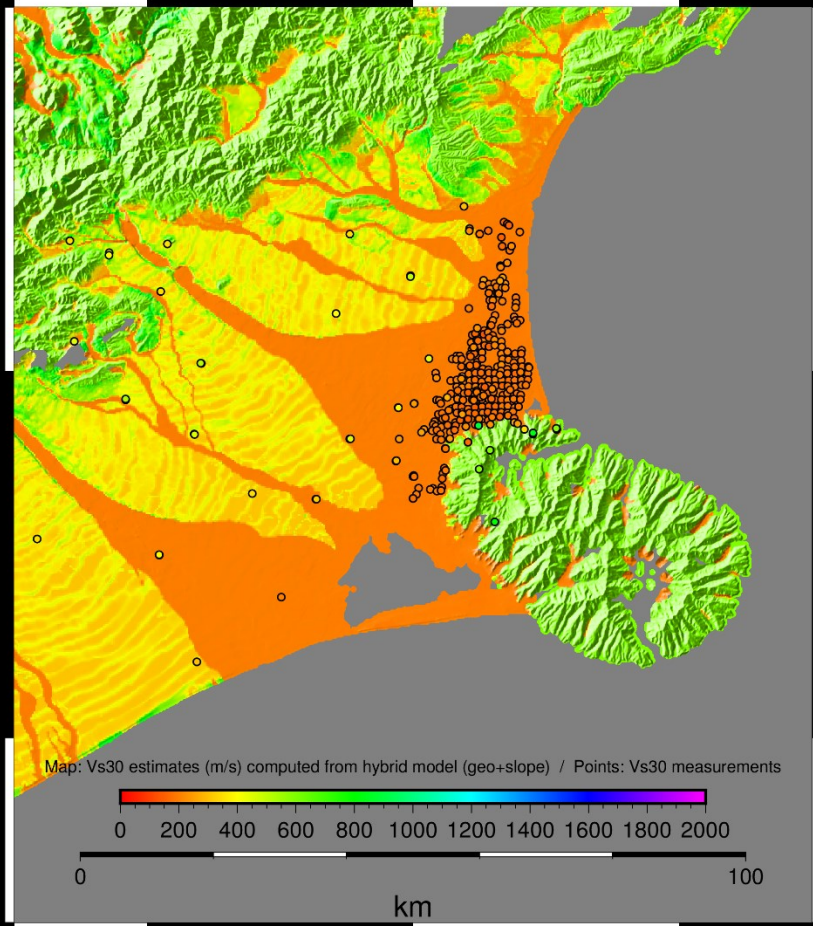
Nonlinear Site Effects in Simulations

- 4 Methods for Incorporating Site Effects:

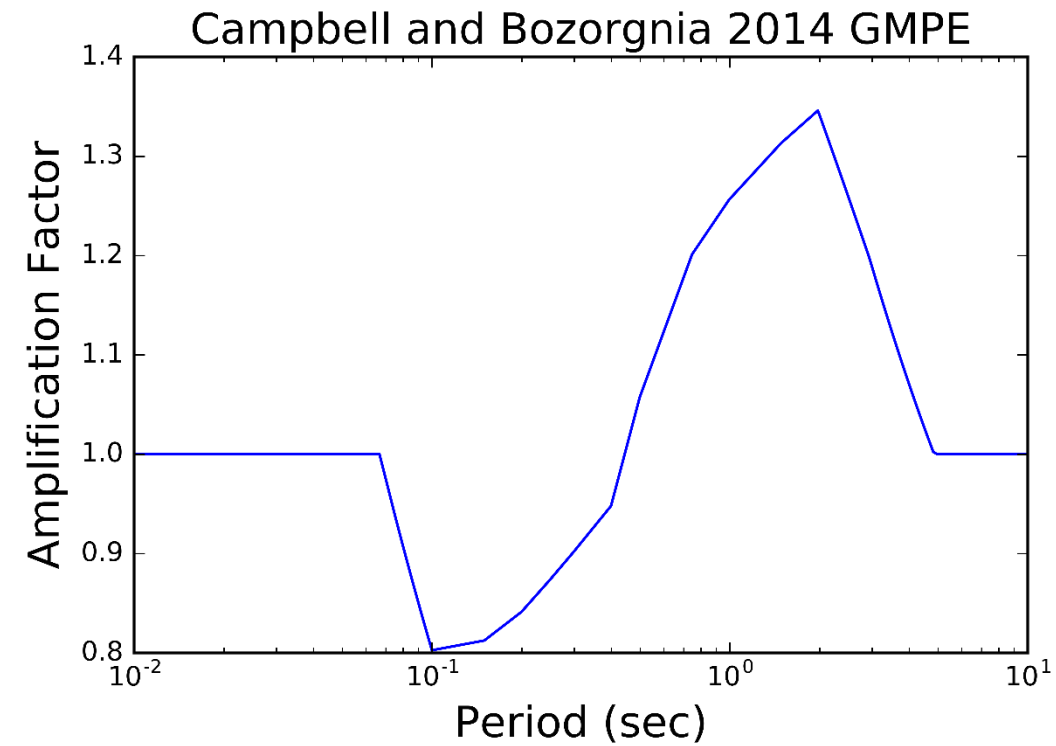
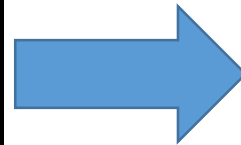
- Fully coupled 3D
- Domain Reduction Method
- Empirical Vs30-based
- Wave propagation site response



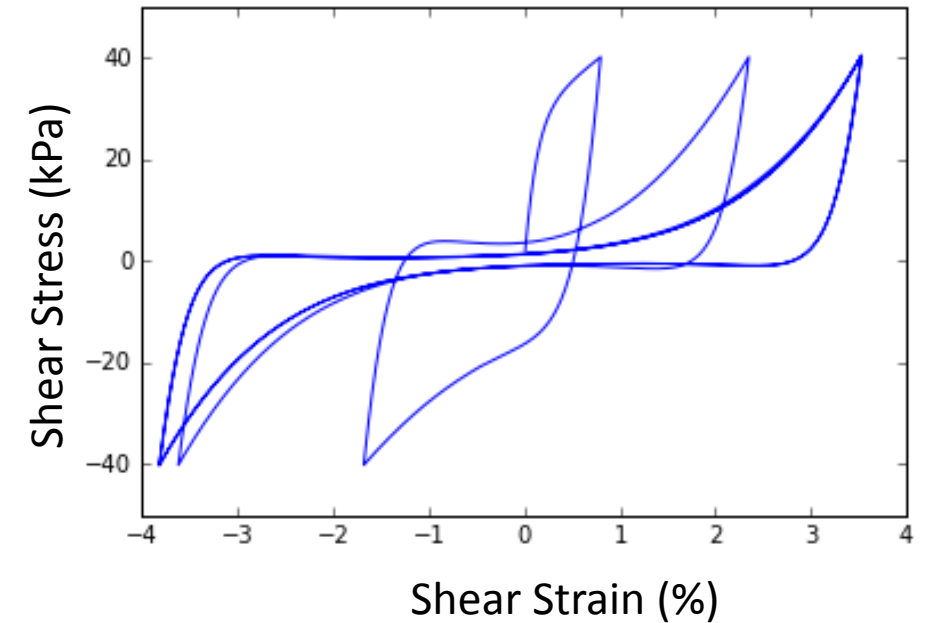
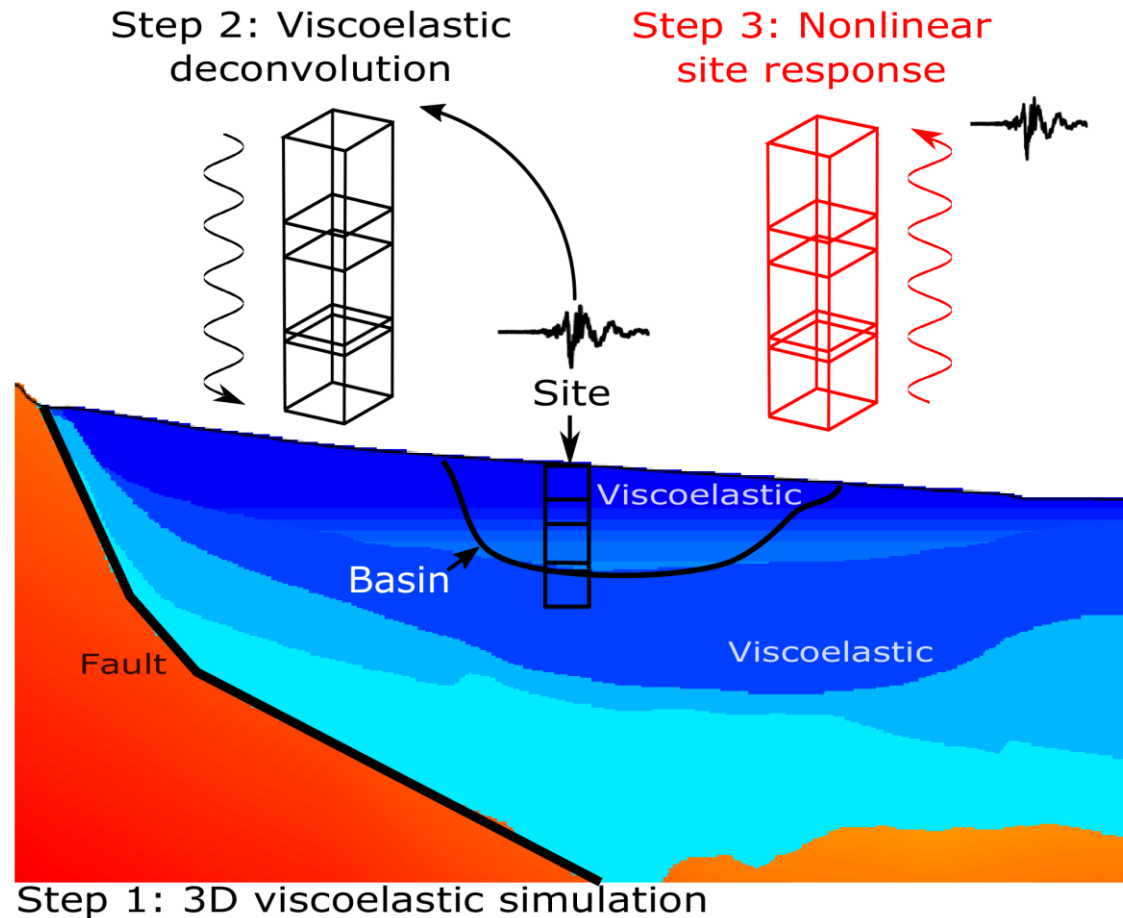
Empirical V_{S30} -Based Factors



V_{S30} , PGA



1D Wave Propagation Site Response



Summary of Previous Studies

- Hartzell et al. 2002

- Mw6.5 Seattle Fault, USA
- Linear, Eq. Linear, Total and Effective Stress Nonlinear

	East-West Acc.	North-South Acc.	East-West Vel.	North-South Vel.	PSA 0.1 Sec North-South	PSA 0.33 Sec North-South	PSA 1.0 Sec North-South	PSA 3.3 Sec North-South
Input Motion*	705	760	75	125	1150	1370	740	455
SHAKE91	1.3	1.1	1.3	1.4	0.8	1.0	1.0	1.6
DESRA2	1.3	1.3	1.3	1.3	0.7	1.1	1.2	1.2
NONLI3 OCR = 3	1.0	1.0	1.2	1.2	1.2	1.1	1.2	1.3
NONLI3 OCR = 1	0.6	0.6	1.2	1.2	0.9	1.0	1.2	1.3
NOAH	0.7	0.6	1.8	1.7	1.1	1.3	1.8	1.8

*All values are in cgs units (cm/sec, cm/sec²)

- Roten et al. 2012

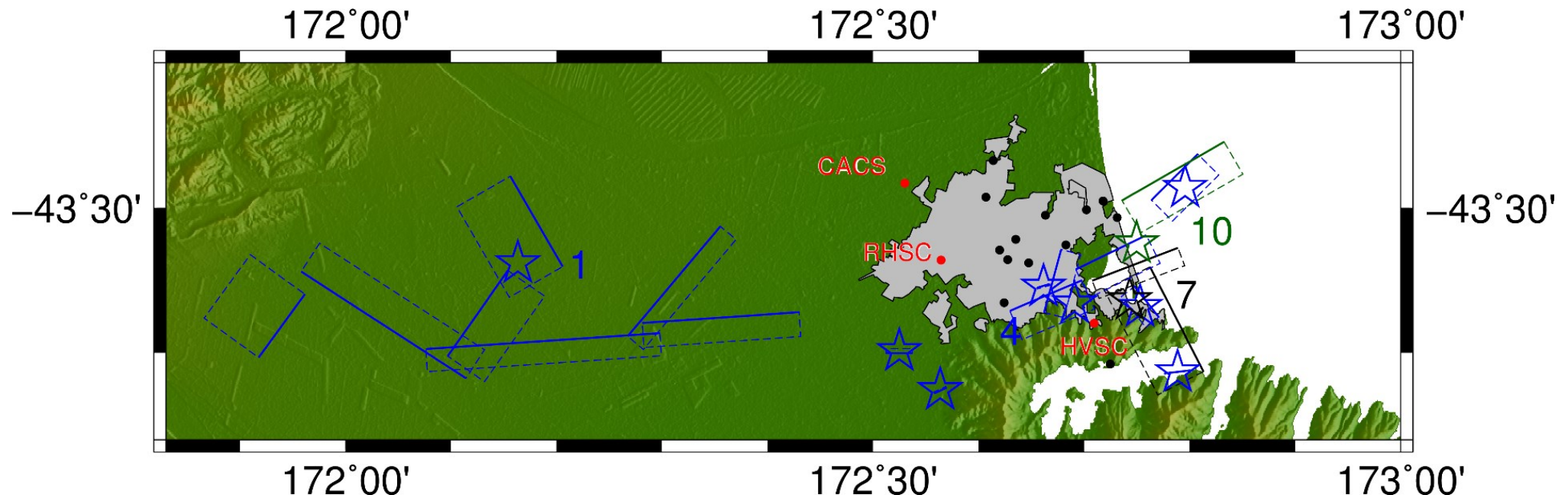
- Mw7.0 Wasatch Fault, Utah, USA
- Total Stress Nonlinear

Outline of Objectives

- Objective 1: Nonlinearity in Simulations of the 2010-2011 Canterbury Eqs
- Objective 2: Effective Stress Site Response for Liquefiable Sites
- Objective 3: Model Uncertainty in 1D Site Response Analysis
- Objective 4: Apply Lessons Learned to Kaikoura Earthquake at Wellington

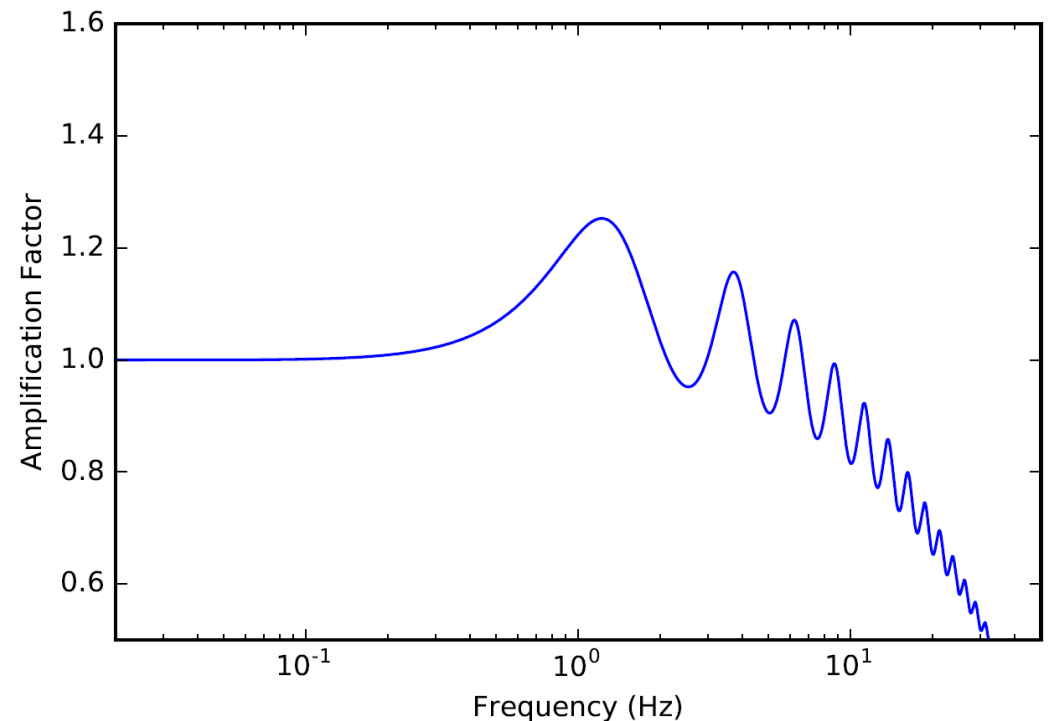
Objective 1: 2010-2011 Canterbury EQs

- Simulations from Razafindrakoto et al 2016.
- 10 events Magnitude 4.7 – 7.1
- 17 strong motion stations in Christchurch
- Total stress site response



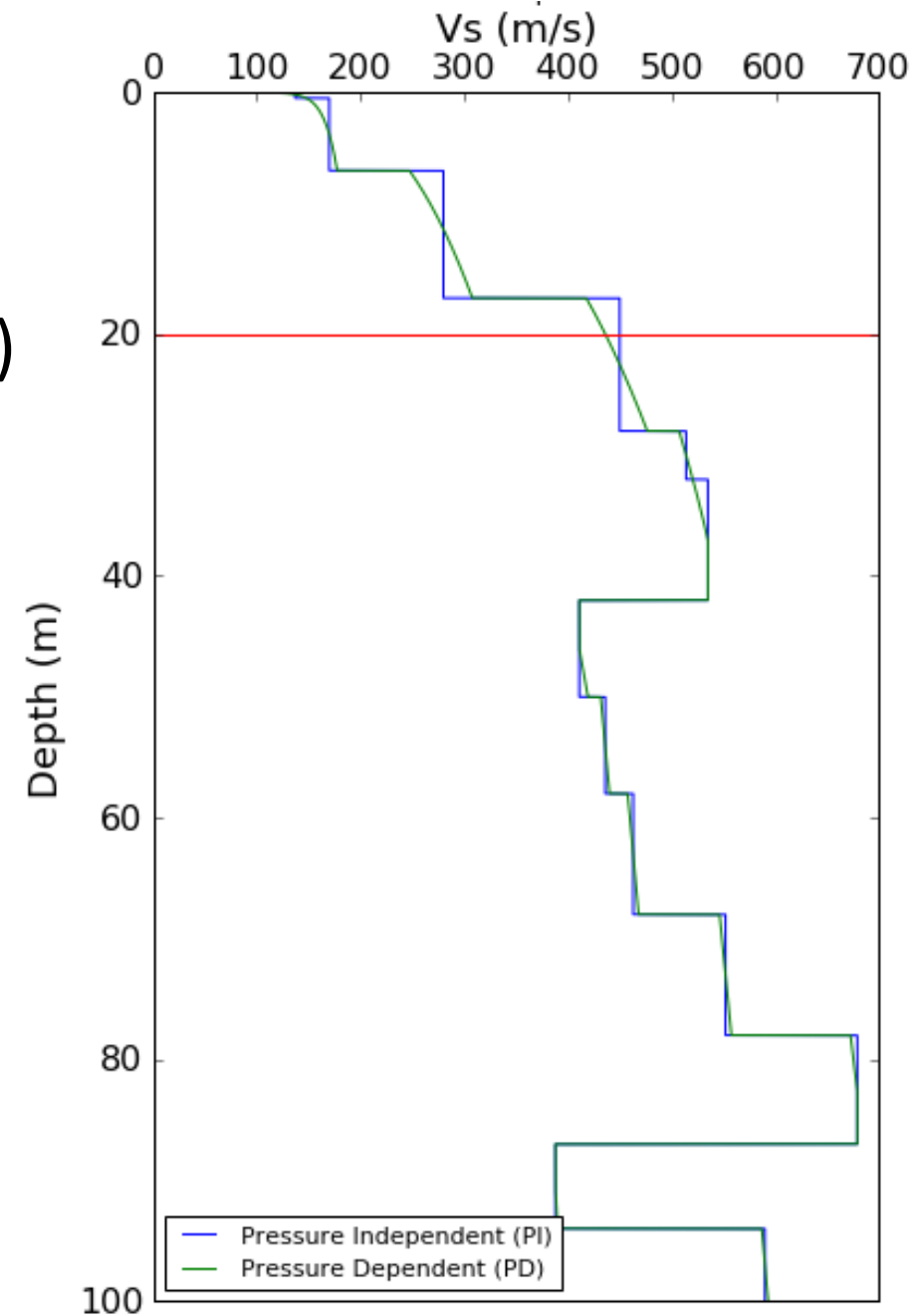
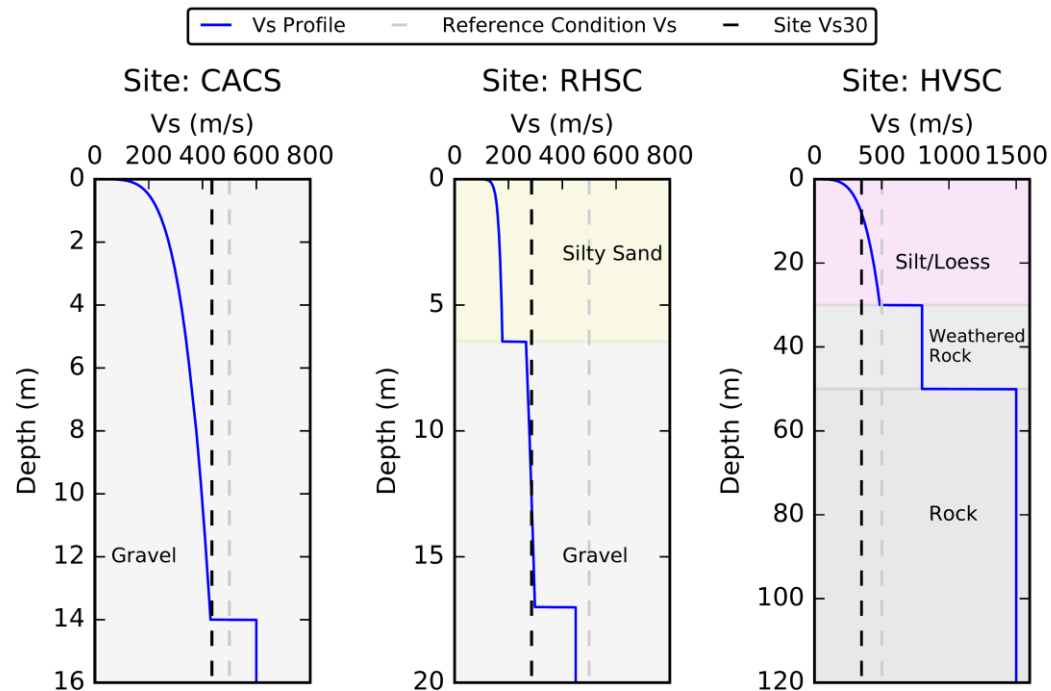
1D Wave Propagation Site Response Analysis

- Deconvolve with frequency domain solution
 - From $V_{S,ref}$ to stiff soil/rock
 - Riccarton gravel: $V_S = 400 - 600$ m/s
- OpenSees FE Code
- PDMY Constitutive Model



Site Characterisation

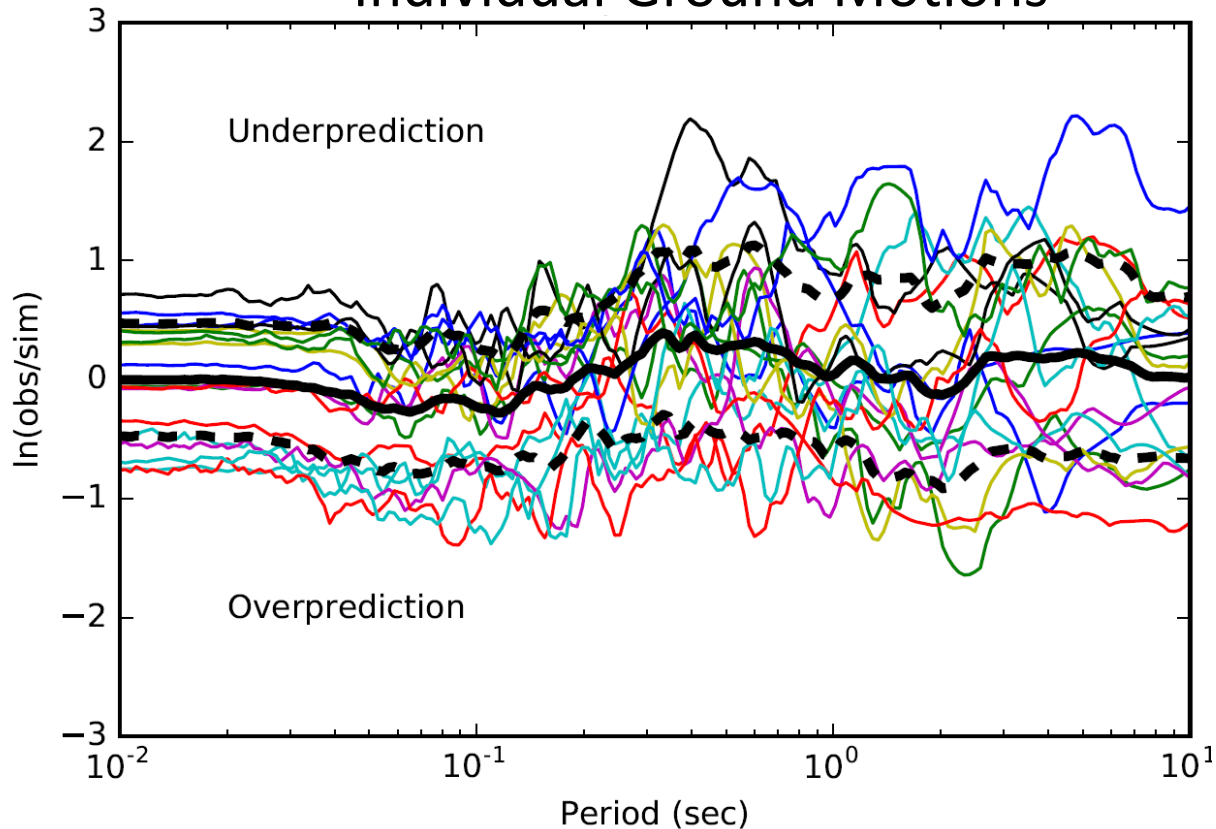
- Wood et al. (2011) and Wotherspoon et al (2014)
- SPT, CPT, V_s
- Deep V_s profiles: Teague et al. 2017



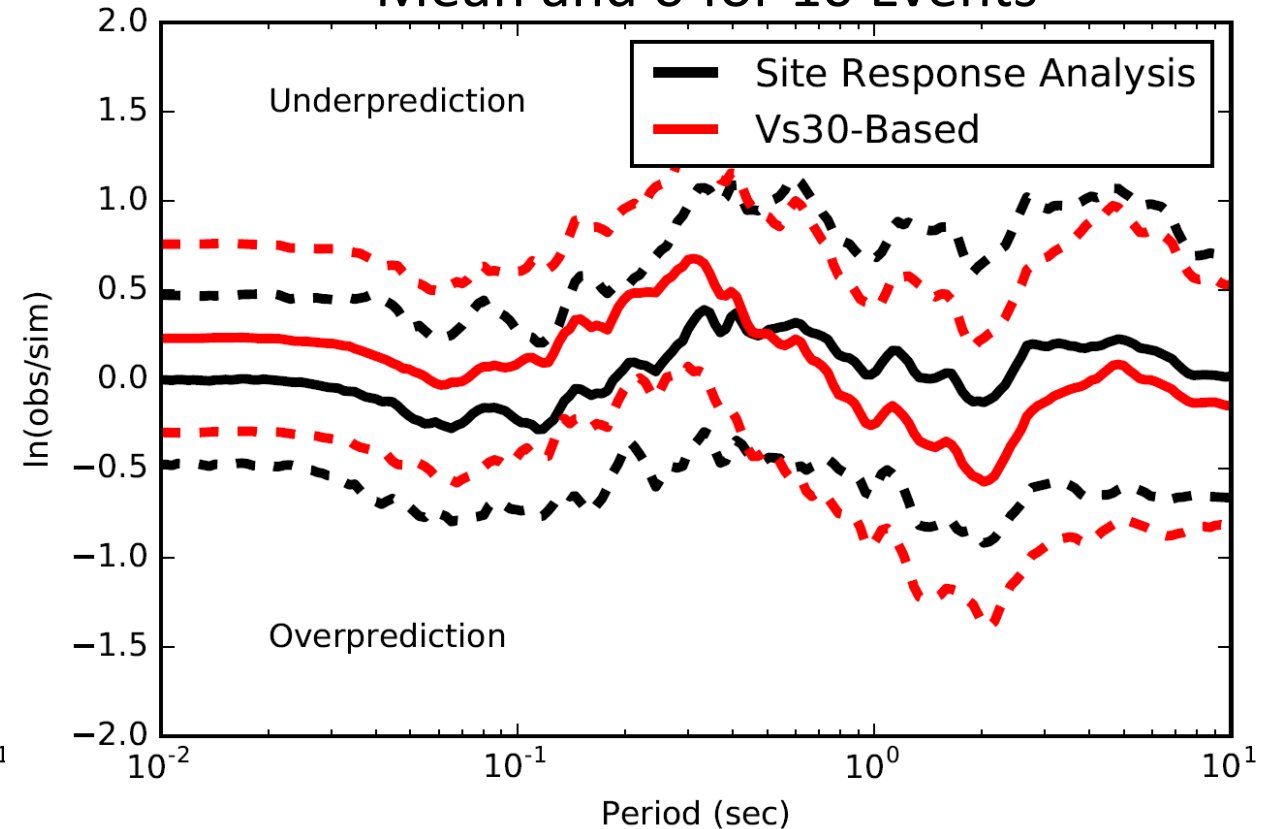
Metric for Quality of Simulations

- Residual = $\ln(\text{PSA}_{\text{Observed}}) - \ln(\text{PSA}_{\text{Simulated}})$

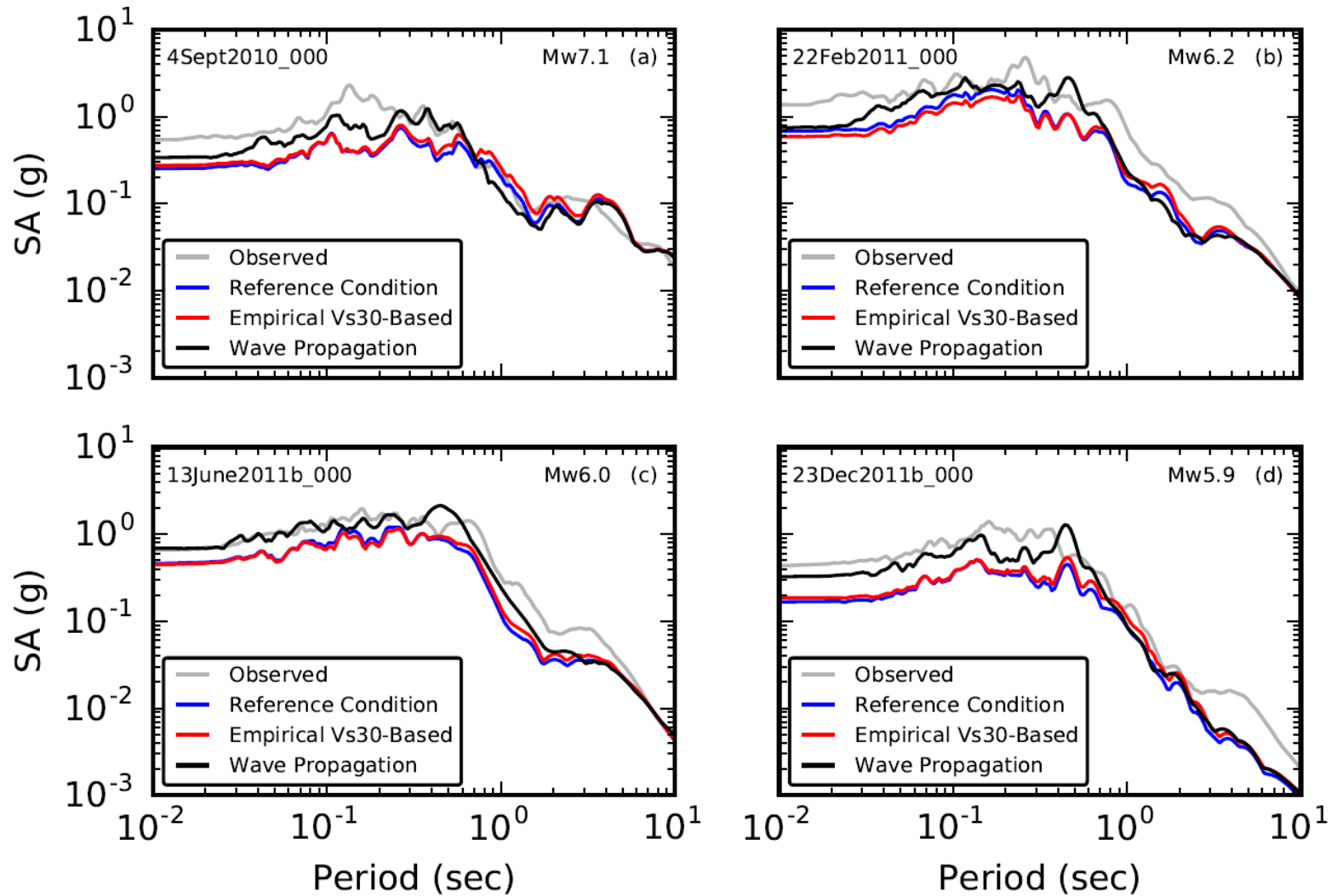
Individual Ground Motions



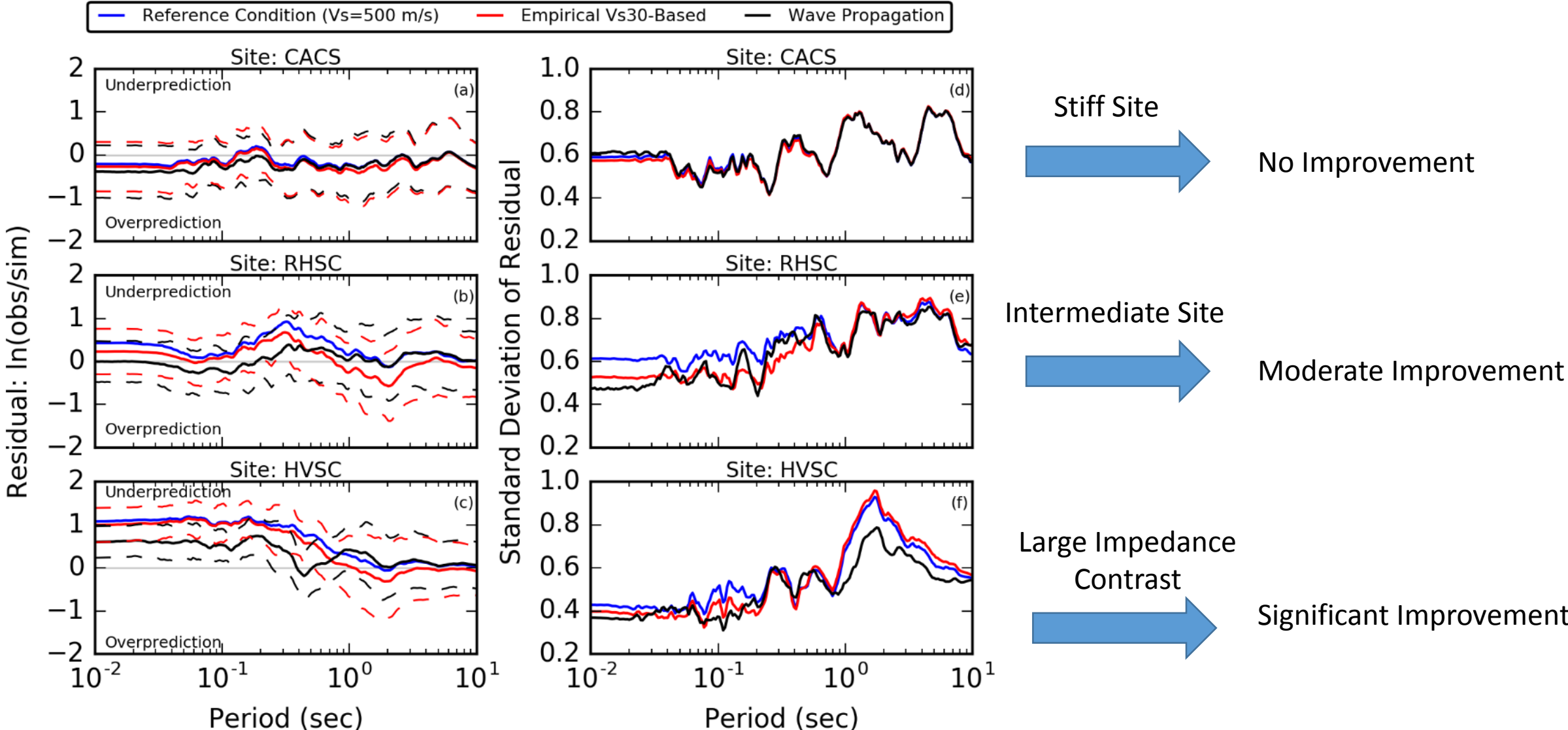
Mean and σ for 10 Events



Comparison of Response Spectra

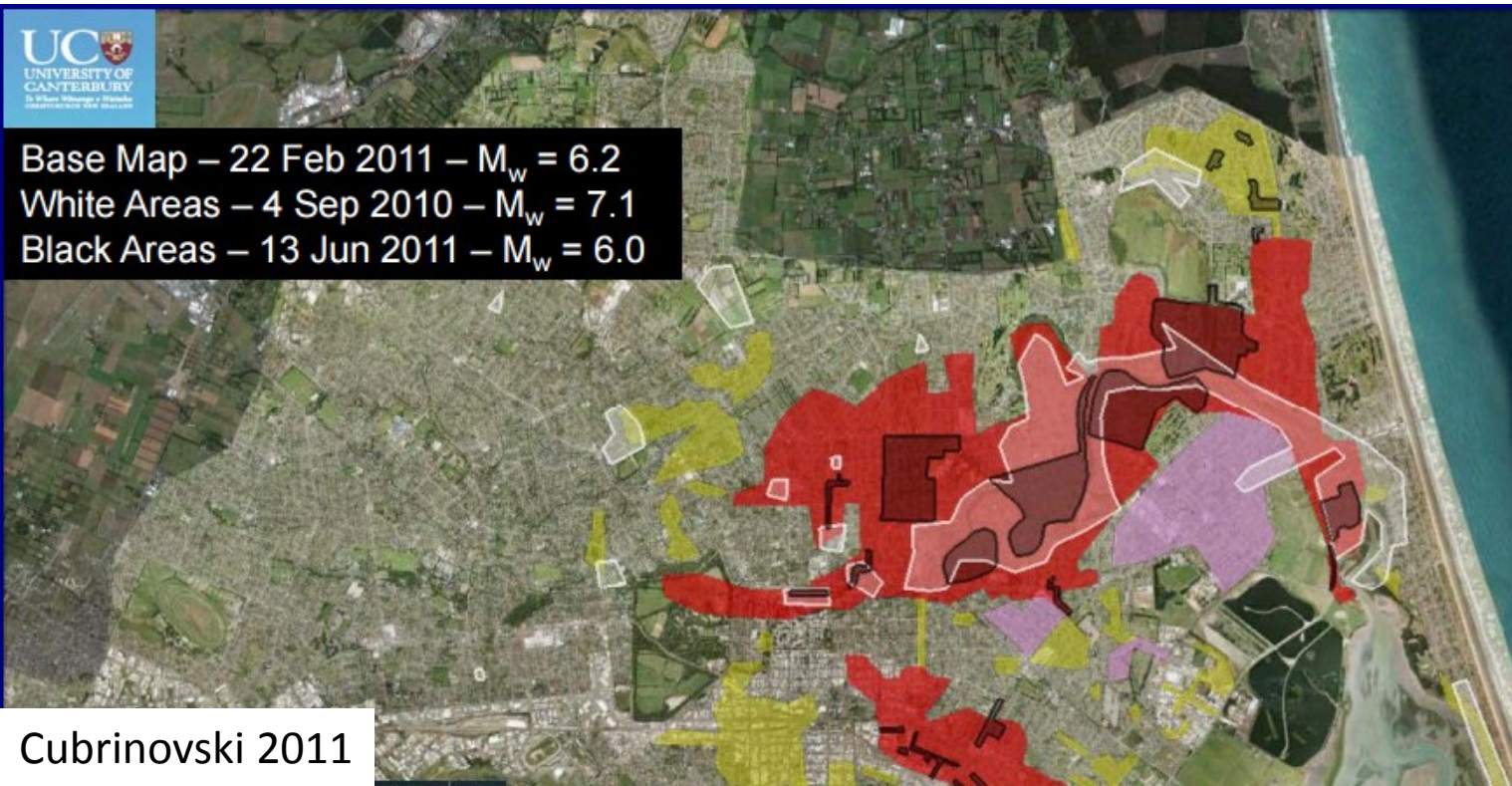


Example Results: 3 Sites



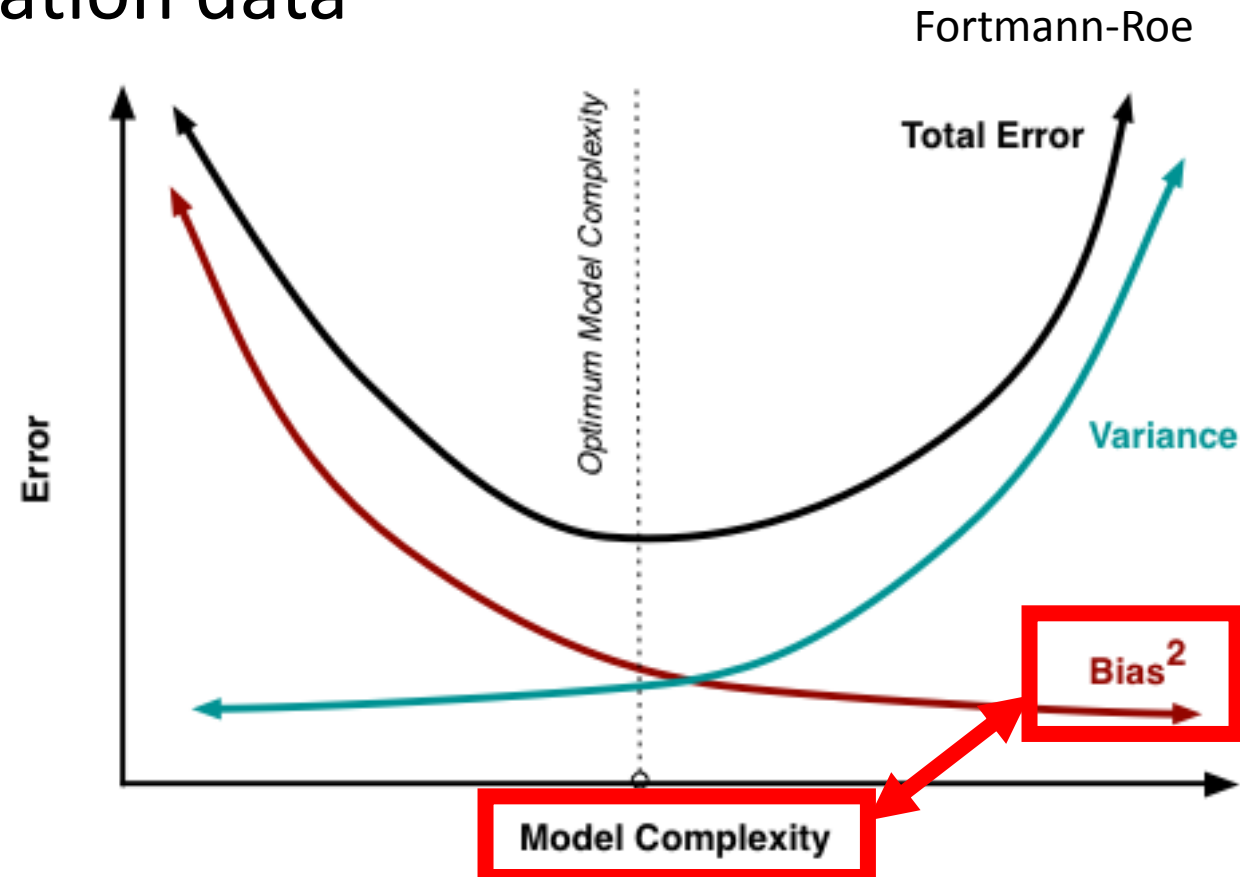
Objective 2: Effective Stress Site Response

- Liquefaction in Mw7.1 Darfield and Mw6.2 Christchurch EQs
- Stress-density constitutive model
- When is Effective Stress $>$ Total Stress ??



Objective 3: Model Uncertainty in Site Response

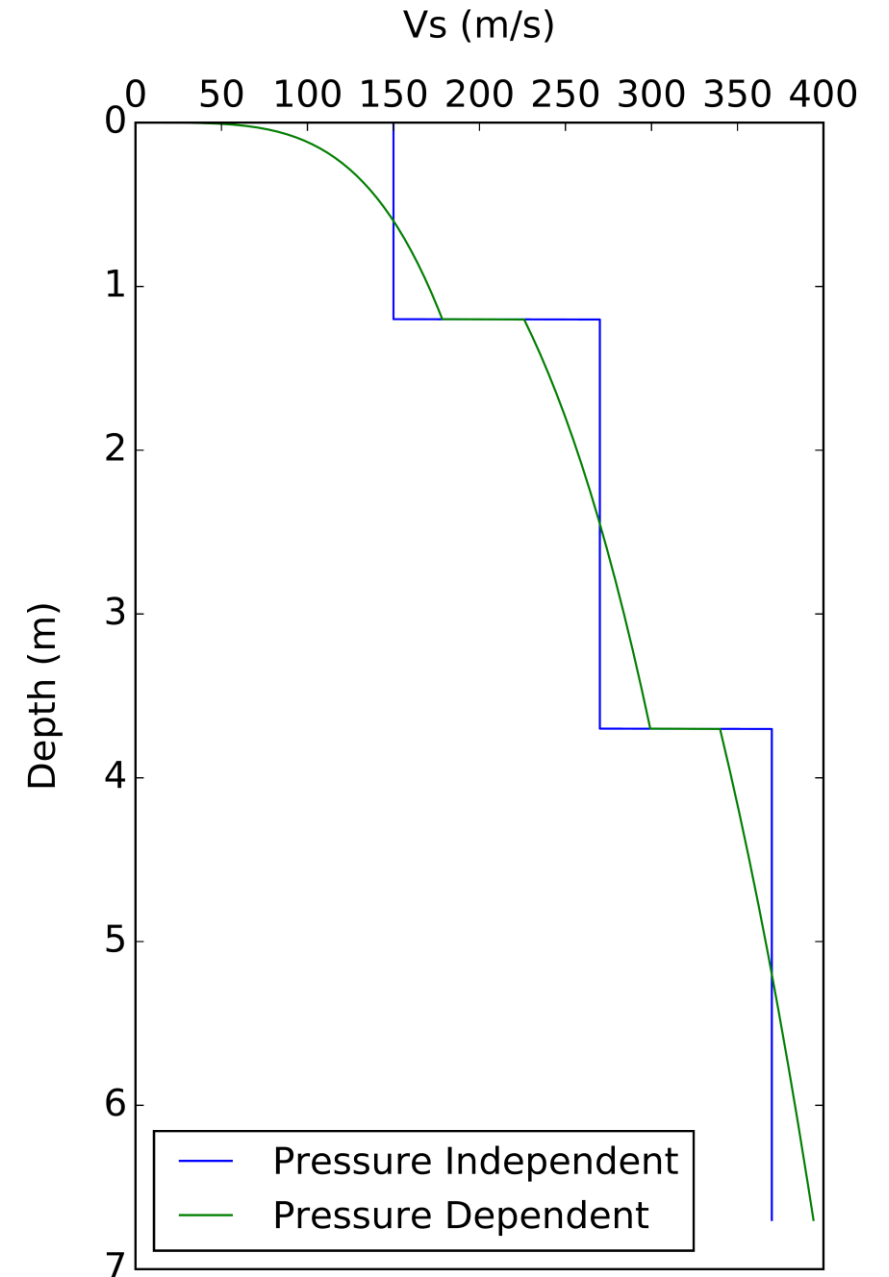
- Can we reduce bias by increasing model complexity??
- Maintain same site characterization data
- Increasing model complexity
 - Pressure Dependent Vs
 - 3D-1D Site Response



Pressure-Dependent V_s

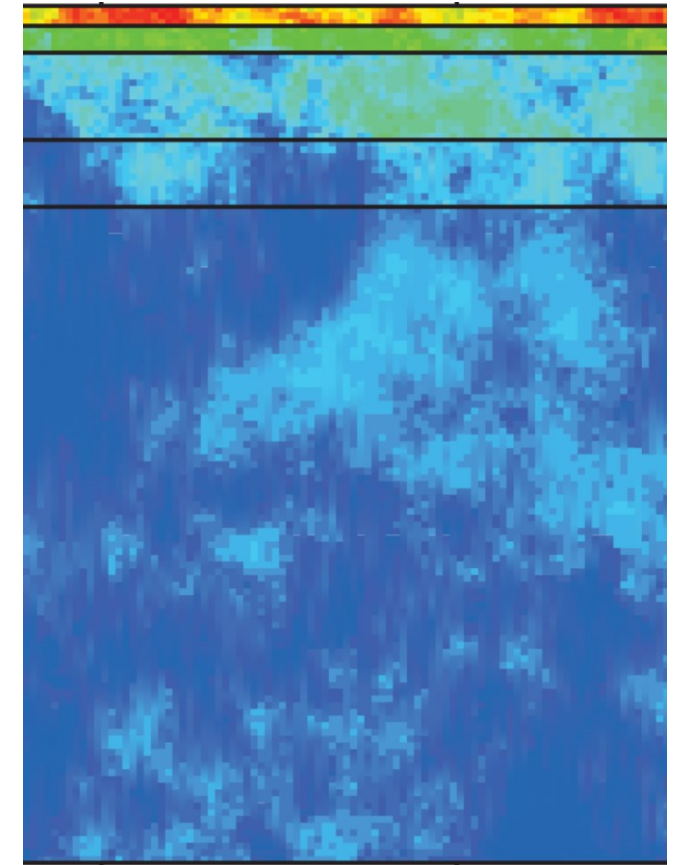
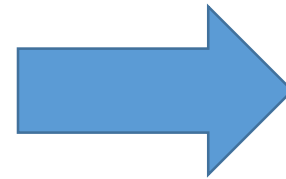
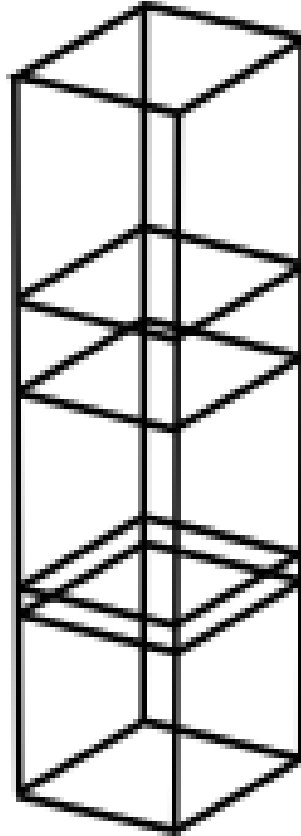
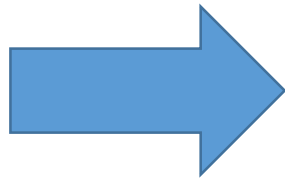
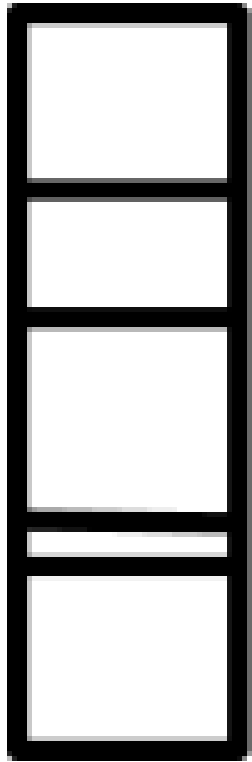
- Published profiles: Constant V_s
- Pressure dependence = depth dependence
- Maintain equal travel time btwn profiles

$$G = G_{ref} * \left(\frac{p'}{p'_{ref}} \right)^d$$



3D-1D Site Response

Thompson et al. 2009



Objective 4: 2016 Mw7.8 Kaikoura EQ

- Severe liquefaction of reclaimed land:
 - Hydraulically-placed dredged fill
 - End-dumped quarry rock



Thank you!

- Acknowledgements:

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